

ASDSO
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GEOSYNTHETICS IN DAMS
FORTY YEARS OF EXPERIENCE

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Geosynthetics:

- GEOMEMBRANES
- GEOTEXTILES
- GEOMATS
- GEONETS
- GEOCOMPOSITES
- GEOGRIDS
- etc.

GEOMEMBRANES



Used as liquid barriers

GEOTEXTILES

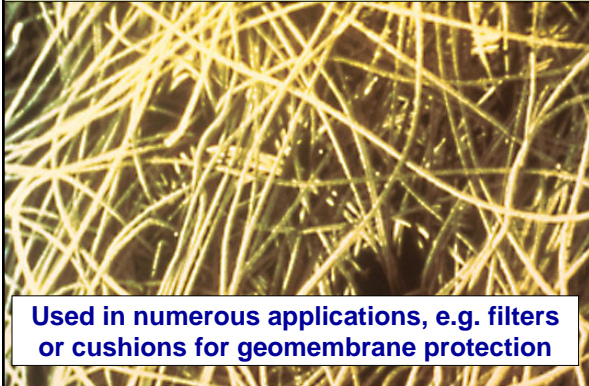


WOVEN GEOTEXTILE

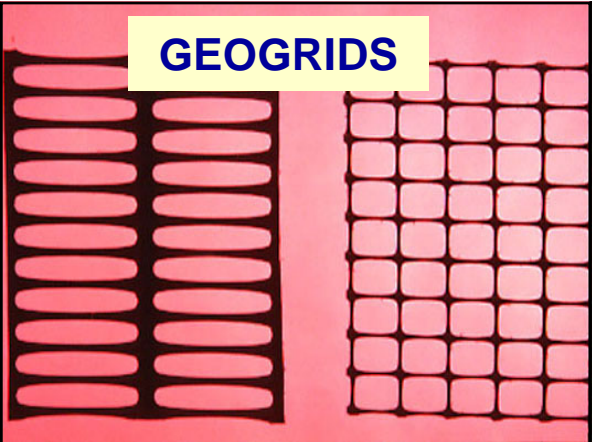
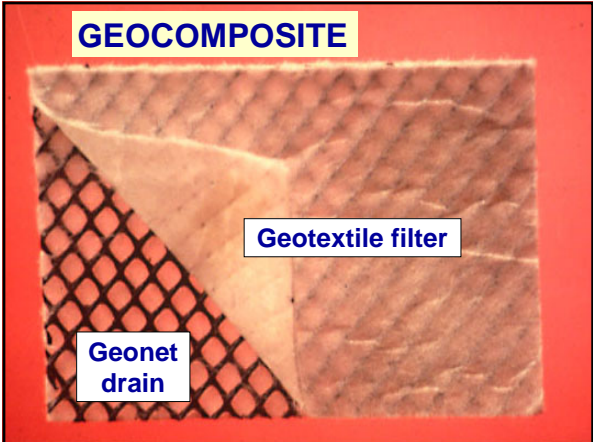
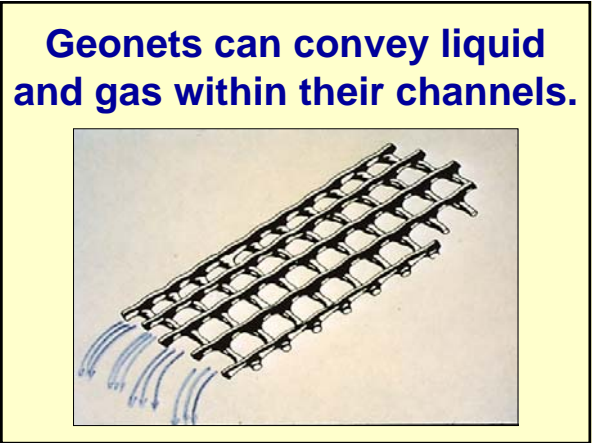
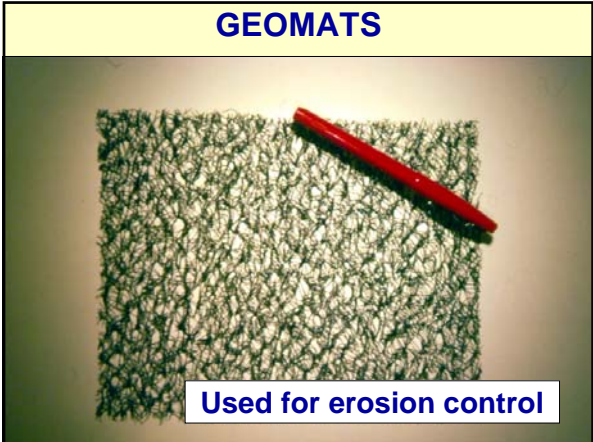


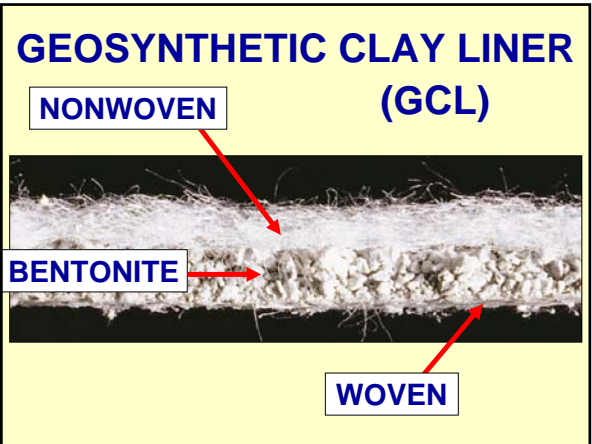
Used as filters or for soil reinforcement

MICROPHOTOGRAPH OF NONWOVEN GEOTEXTILE



Used in numerous applications, e.g. filters or cushions for geomembrane protection





All types of geosynthetics are used in dams.

Geosynthetics can perform a **variety of functions** in dams and can be used at a **variety of locations** in a dam.

APPLICATIONS OF GEOSYNTHETICS IN DAMS

- Water barrier (GEOMEMBRANE)
- Internal filter (GEOTEXTILE)
- Drainage (GEOCOMPOSITE)

These are the essential functions, the functions involved in seepage control.

APPLICATIONS OF GEOSYNTHETICS IN DAMS

- Reinforcement (GEOGRID, GEOTEXTILE)
- Bank protection (GEOTEXTILE)
- Erosion control (GEOMAT, GEOCELL)
- Cushioning (GEOTEXTILE)

These functions/applications are important, but not essential.

GEOMEMBRANE BARRIERS IN DAMS

CONTRADA SABETTA DAM (ITALY) 1959

- 32.5 m high (106 ft), 1H/1V
- Dry masonry
- Polyisobutylene geomembrane
- 2 mm thick
- Underlain by drainage layer
- Protected by concrete slabs, 2 m x 2 m x 0.2 m with 1 mm joints.

CONTRADA SABETTA DAM



CONTRADA SABETTA DAM




- Samples of the geomembrane were taken 38 years after construction.
- Polyisobutylene geomembrane still in good condition.

In 1960, Terzaghi used a PVC membrane at Mission Dam (now Terzaghi Dam), with some problems.




MIEL DAM (1968) 13 m (43 ft)




Butyl rubber, 1 mm

L'OSPEDALE DAM, 85 ft (1978)
 Bituminous geomembrane, 4 mm



L'OSPEDALE DAM, 85 ft (1978)



L'OSPEDALE DAM

GEOMEMBRANE CONNECTION AT TOE BEAM

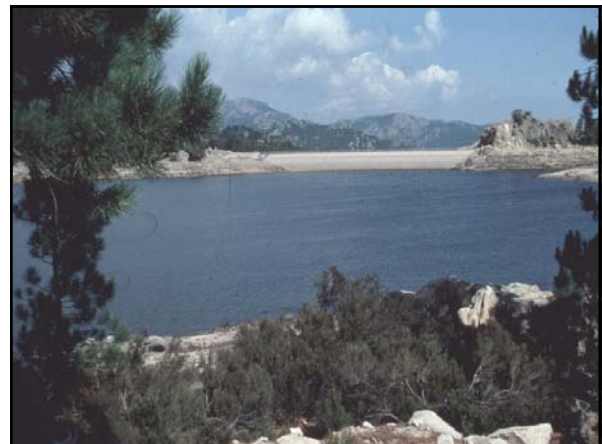
BATTEN STRIP




CONCRETE PAVER BLOCKS FOR GEOMEMBRANE PROTECTION

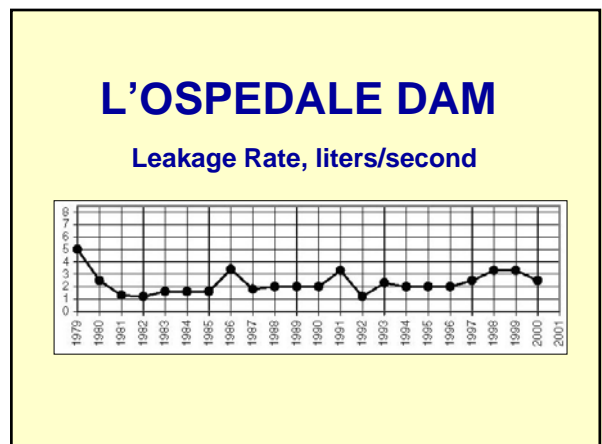
INTERLOCKING PAVERS:
 8 in. long
 3 in. thick

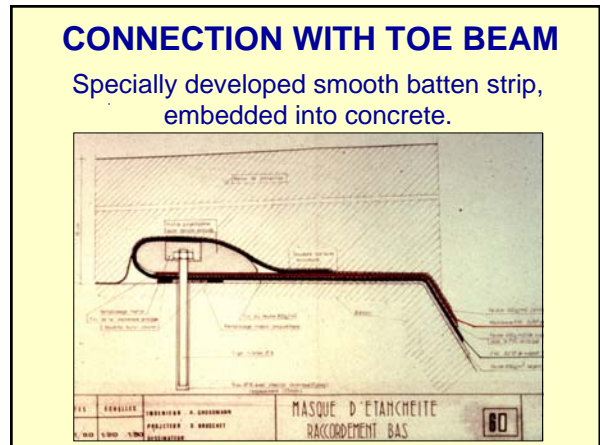
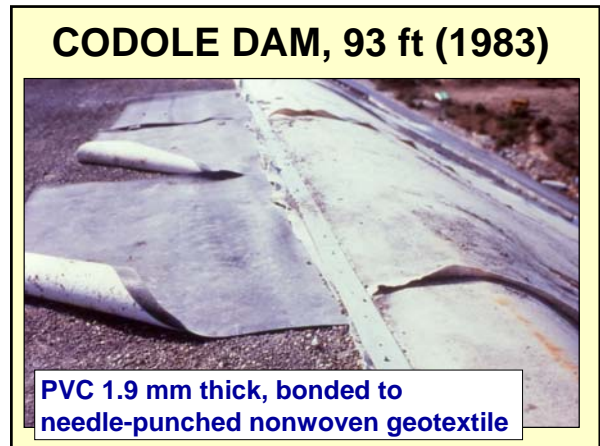
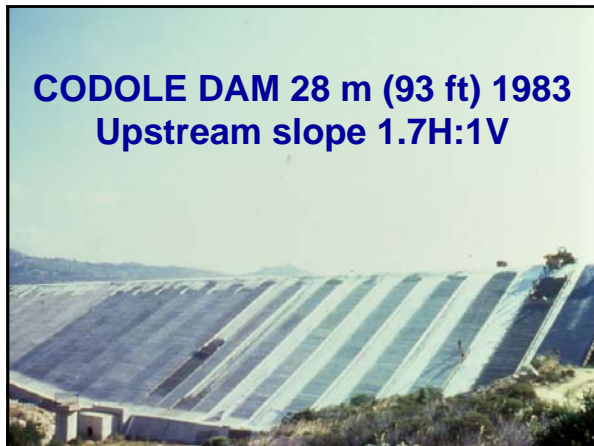


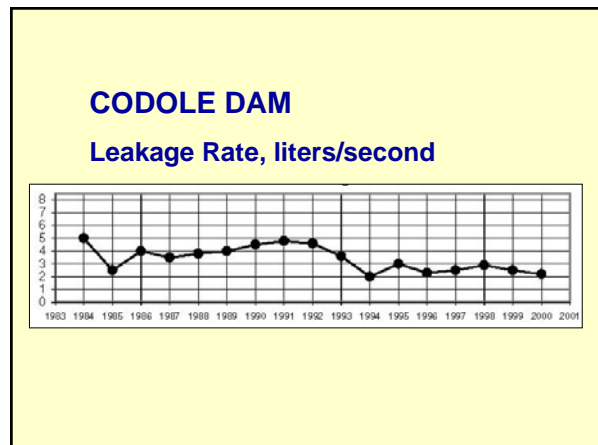
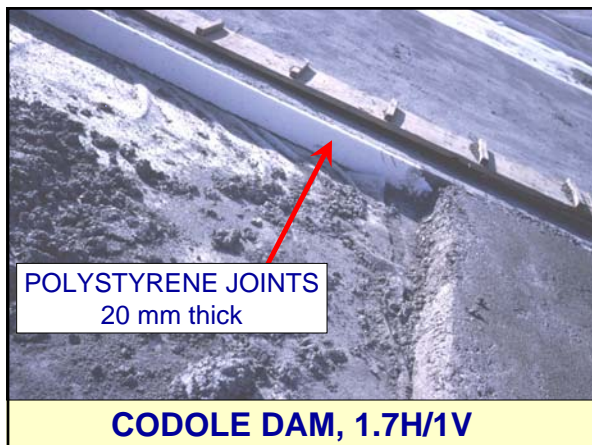



CONCLUSION OF THE INCIDENT

- The geotextile cushion between the geomembrane and the paver blocks performed its function and the **geomembrane was not damaged.**
- Repair was done with the same interlocking blocks and concrete.
- In subsequent dams, **concrete slabs** will be preferred.








- FIGARI DAM**
CONSTRUCTION PROBLEMS
- The PVC geomembrane was **independent** from the underlying geotextile.
 - This made installation easier, but caused two **major problems**.
 - The **geomembrane crept** down the slope during construction.
 - The geomembrane was uplifted by the wind with no damage but the **geotextile was displaced** under the geomembrane.
 - The geomembrane had to be removed to reposition the geotextile, and 50% of the geomembrane had to be discarded.

**FIGARI DAM
 LESSON LEARNED**

- Non-reinforced PVC geomembranes should not be used on steep slopes.
- If geomembrane **uplift by wind** is likely to occur during construction, the geotextile underlying the geomembrane should be either **bonded** to the geomembrane or **bonded** to the underlying material (assuming this material is rigid).

BORFLOC'H DAM 19 m (62 ft) 1993

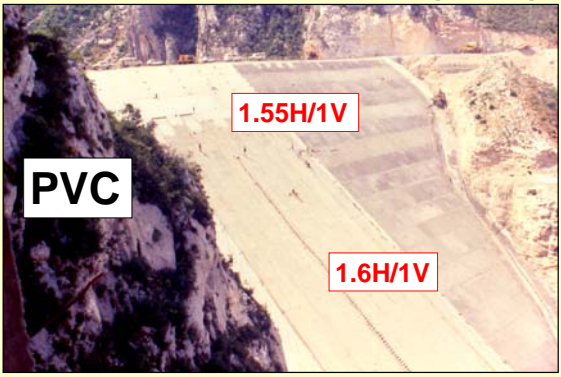


Bituminous geomembrane 4 mm thick, bonded to the underlying permeable asphalt concrete (one point every 3 m)

BORFLOC'H DAM



BOVILLA DAM, 187 ft (1996)



BOVILLA DAM

- PVC geomembrane, **3 mm** thick, heat-bonded to a polypropylene continuous filament needle-punched nonwoven geotextile, 700 g/m².
- Interface friction angle between this geotextile and granular material support is 38°: factor of safety of 1.2 for the 1.55H/1V slope.

**BOVILLA DAM
 CONCRETE PROTECTION**

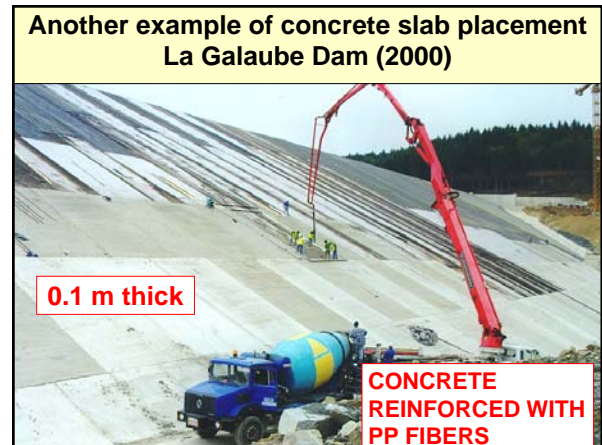
- Protection by cast-in-place **unreinforced** concrete.
- Thick (0.2 m) protection due to risk of rock fall from the sides.
- Concrete thickness 0.3 m in lower central face where slab length exceeds 50 m.

BOVILLA DAM
CONCRETE PROTECTION STABILITY

- Independent polypropylene continuous filament needle-punched nonwoven geotextile, 800 g/m² between geomembrane and concrete.
- Interface friction angle 22° between this geotextile and the geomembrane.
- Slope angle: 33° at top and 32° at toe.
- Concrete slab supported by peripheral beam at toe.

BOVILLA DAM
CONCRETE PROTECTION DRAINAGE

- Joints along the slope between concrete panels to release pressure in case of **rapid drawdown**.
- Joints along the slope filled with three layers of polypropylene continuous filament needle-punched nonwoven geotextile, 350 g/m².
- Horizontal joints between concrete panels filled with one layer of same geotextile to ensure flexibility of concrete slab.



Concrete Slab Protection

| Dam | Year | Height (m) | Slope | Thickness (cm) | Reinforcement |
|------------------|------|------------|---------|----------------|---------------|
| Contrada Sabetta | 1959 | 32 | 1.0H/1V | 20 | None |
| Codole | 1983 | 28 | 1.7H/1V | 14 | Steel mesh |
| Figari | 1991 | 35 | 1.7H/1V | 14 | PP fibers |
| Ortolo | 1993 | 37 | 1.7H/1V | 14 | PP fibers |
| Galaube | 2000 | 42 | 2.0H/1V | 10 | PP fibers |
| Bovilla | 1996 | 81 | 1.6H/1V | 20 | None |

Other possibilities:


- Interlocking concrete blocks (poor performance in large dams)
- Articulated concrete blocks (small dams)
- Soil and rock protection (slopes less steep than 2H/1V)
- No protection (anchorage/bonding against wind uplift)

ARTICULATED CONCRETE BLOCKS WITH STEEL CABLES

Block thickness 12 cm

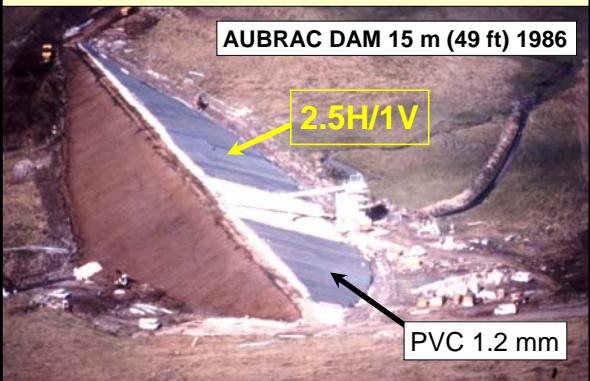
MAURIAC DAM
 20 m high (1989)

MUD LAKE DAM (NEVADA), 2H/1V (2000)



3 in. deep geocell filled with concrete, on geotextile (16 oz/sq.yd) and reinforced polypropylene geomembrane

A dam with soil and rock protection



AUBRAC DAM 15 m (49 ft) 1986

2.5H/1V


PVC 1.2 mm

AUBRAC DAM 15 m (49 ft) 1986
 2.5H/1V

- 0.5 m (20 in.) rockfill 100-300 mm (4-12 in.)
- 0.2 m (8 in.) gravel 0-25 mm (0-1 in.)
- needle-punched nonwoven geotextile (500 g/m²) (15 oz/sq.yd)
- 1.2 mm (47 mil) PVC geomembrane
- needle-punched nonwoven geotextile (500 g/m²) (15 oz/sq.yd)
- 0.2 m (8 in.) drainage layer, gravel 0-25 mm (0-1 in.)

AUBRAC DAM

Slide of the soil cover over 1000 m² during placement of soil cover at interface between PVC geomembrane and underlying geotextile.




AUBRAC DAM 15 m (49 ft) 1986

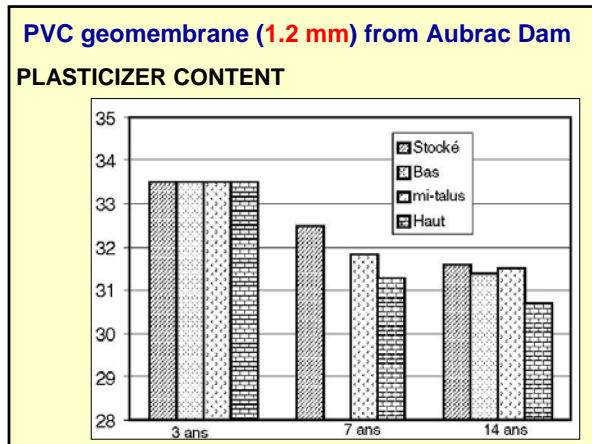
This slide has not been well explained

- **Interface friction angle:**
 shear box (high normal stress) 34°
 inclined plane (low normal stress) 28°
- **Slope angle 22°**
- **Influence of moist interface: -3°**
 Influence of vibrations: -3°

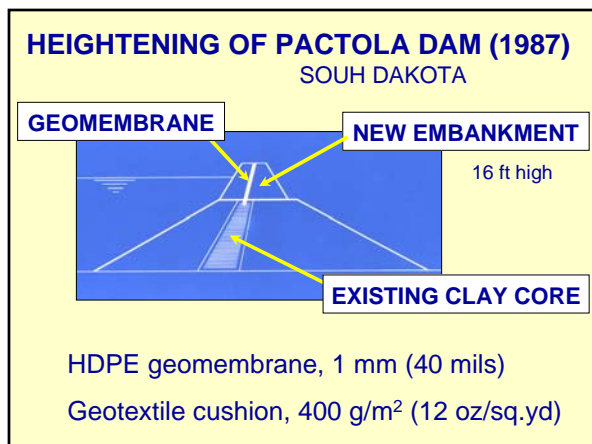
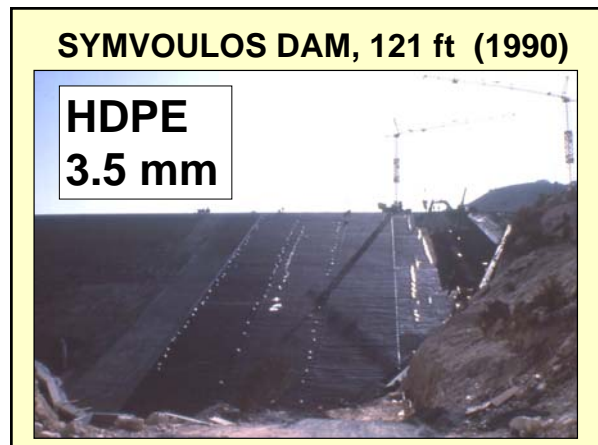
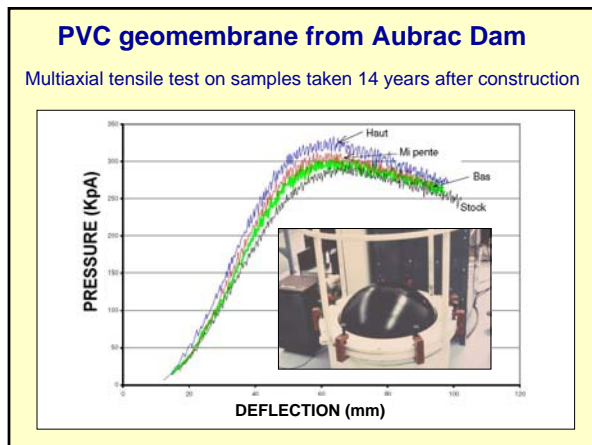
Sample of PVC geomembrane taken from Aubrac Dam

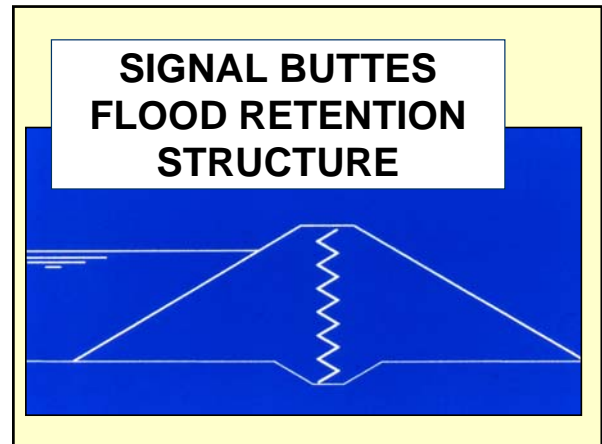
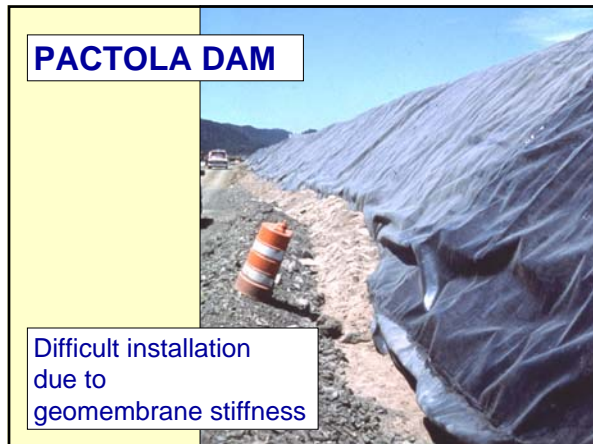
(From coupons placed inside the reservoir under same conditions as in the dam)





- PVC geomembrane from Aubrac Dam**
(1.2 mm thick geomembrane covered with soil)
- Plasticizer loss is limited.
 - Plasticizer loss is greater above water level than below.
 - The rate of plasticizer loss seem to decrease after a few years.
 - Plasticizer loss is about the same for geomembrane below water level and geomembrane in storage.
- and the thickness was only 1.2 mm (47 mils)





In spite of construction problems , Signal Buttes Flood Retention Structure is considered in better condition than conventional homogeneous flood retention structures in the same district, which are generally cracked.

Using a geomembrane is clearly the best solution for earth dikes that are susceptible to cracking because they are not exposed to water most of the time.

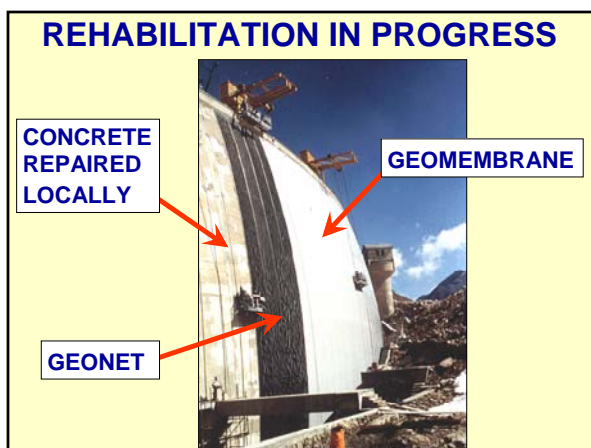
The Signal Buttes Flood Retention Structure is a rare example of dam with internal geomembrane.

A multi-stage construction is a better strategy than the accordion shape.

STAGE 4
STAGE 3
STAGE 2
STAGE 1



**USE OF
GEOSYNTHETICS
TO REHABILITATE
OLD CONCRETE DAMS**



REHABILITATION CONCEPT

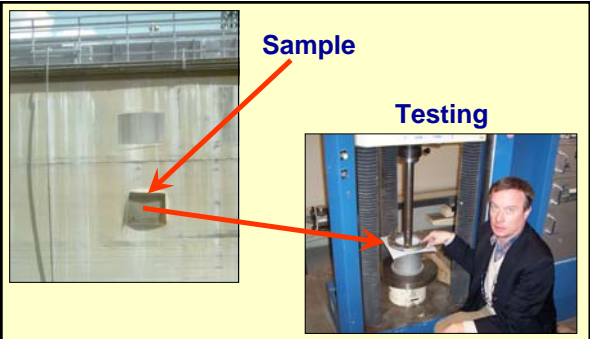
- The geomembrane provides impermeability.
- A **geonet** or a thick **geotextile** placed between the geomembrane and the concrete is acting as a **drain**.
- The main purpose of the system is to allow the concrete to progressively dry.
- **Removing water from concrete** decreases to a negligible level frost action and alkali-aggregate reaction.
- The geomembrane also decreases to a negligible level the **leakage** associated with concrete deterioration.

DURABILITY

- Durability is a major consideration in this application.
- In the rehabilitated dams, the concrete had deteriorated to a critical level in 40-60 years.

GEOSYNTHETIC DURABILITY

- In this application, the geosynthetics are exposed to harsh conditions (sunlight, weather, floating debris).
- To ensure durability, the geosynthetics have been carefully selected.
- To check durability, the geosynthetics are tested periodically.



Based on 20 years of testing, a durability of 50 years is predicted.

SYSTEM DURABILITY

- The geosynthetics on the dam face can be **easily replaced** at the end of their service life.
- This increases the durability of the dam **indefinitely**.

A good example of complementarity between geosynthetics and traditional construction materials

More than 150 large dams worldwide (including 90 in Europe) have been constructed with a geomembrane.

DISTRIBUTION IN EUROPE

| Geomembrane | Total | New | Repair |
|-------------|-------|-----|--------|
| PVC | 55% | 20% | 35% |
| Bituminous | 15% | 12% | 3% |
| In situ | 11% | | |
| Elastomers | 5% | | |
| Others | 14% | | |

PVC geomembrane used on all types of dams.
Bituminous geomembrane used mostly on embankment dams.

Drainage is present under geomembrane in most cases. →

In all the large dams where a geomembrane is used on the upstream face, the **geomembrane is the only line of defense.**

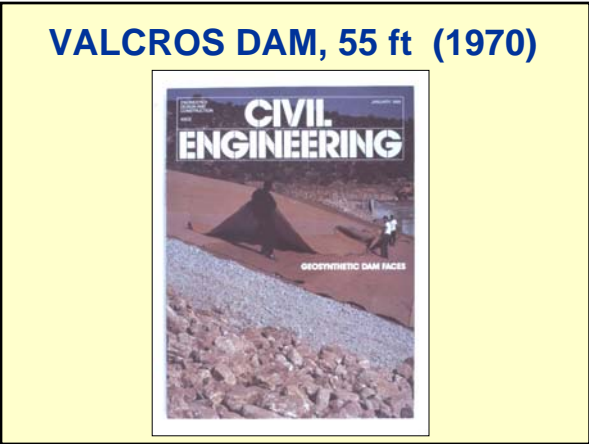
and the resulting leakage rate is very small →

LEAKAGE THROUGH DAMS

| Dam | Height (m) | GM | Thick (mm) | Leakage (l/hr/m ²) |
|-----------|------------|------|------------|--------------------------------|
| Ospedale | 26 | Bitu | 5 | 2.4 |
| Codole | 28 | PVC | 2 | 0.9 |
| Mauriac | 14 | Bitu | 4 | 0.9 |
| Figari | 35 | PVC | 2 | 1.0 |
| Madone | 18 | PVC | 2 | 0.3 |
| Borfloc'h | 19 | Bitu | 4 | 1.3 |
| Empurany | 19 | PVC | 1 | 11.0 |
| Ortolo | 37 | Bitu | 5 | 7.0 |

- COMMENTS ON LEAKAGE RATES
- Observed leakage rate of the order of **1 liter/hr/m²** (if accurate).
 - One defect per 1000 m² with a diameter of 2 mm would give a leakage rate of the order of **0.1 liter/hr/m²**.
 - Leakage at connections may explain the difference.

GEOTEXTILE FILTERS IN DAMS



VALCROS DAM, 55 ft (1970)



VALCROS DAM

DESCRIPTION

- 55 ft high
- Homogeneous dam
- Silty sand, 30% < 0.075 mm (#200 sieve)
- Polyester continuous filaments needle-punched nonwoven geotextile, 300 g/m² (9 oz/sq.yd)



VALCROS DAM

PERFORMANCE

- Constant **trickle of clean water** for 35 years (traces of suspensions were noticeable in the water only for a few days after filling of the reservoir)
- **Less than 0.1 liter/hr/m²** of dam (which is consistent with the hydraulic conductivity of the embankment soil, $k = 1 \times 10^{-7}$ m/s)
- **No seepage** of water ever observed through the downstream slope.

GEOTEXTILE DURABILITY

Samples of geotextile have been removed from the actual filter, 6 years and 22 years after construction.



VALCROS DAM
DOWNSTREAM
SLOPE

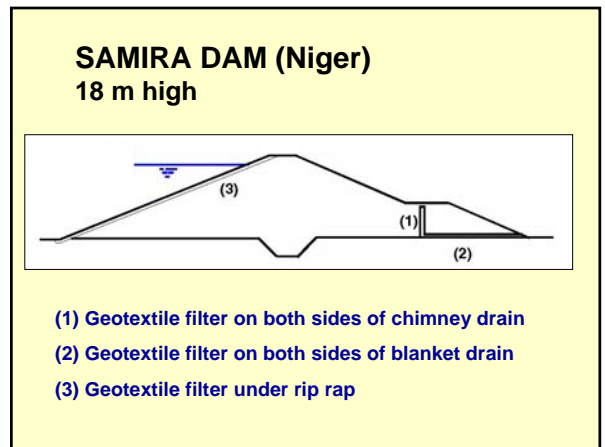
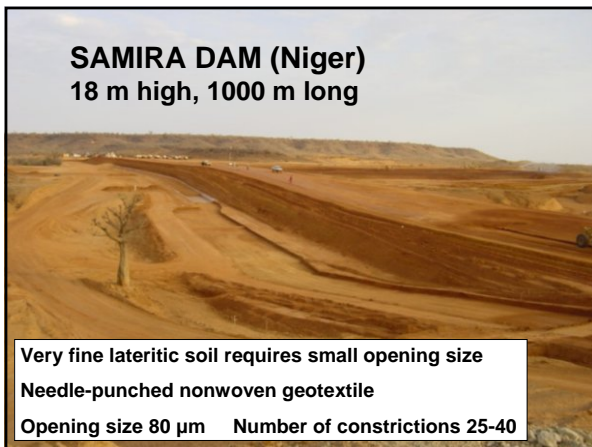
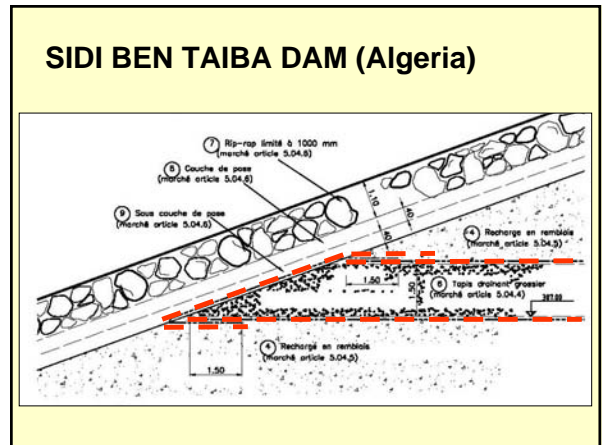
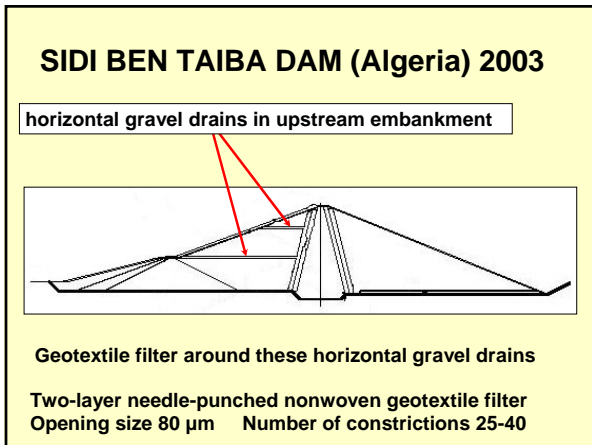
VALCROS DAM

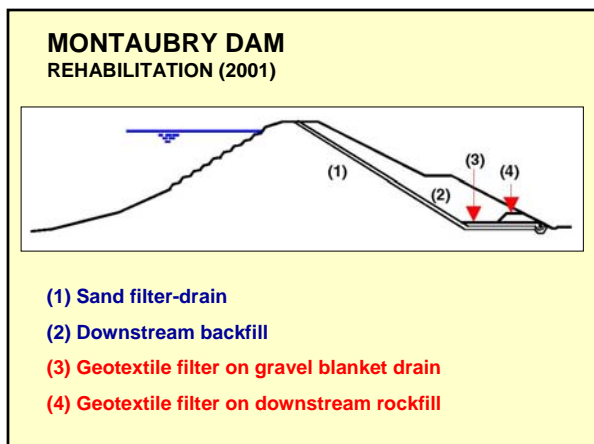
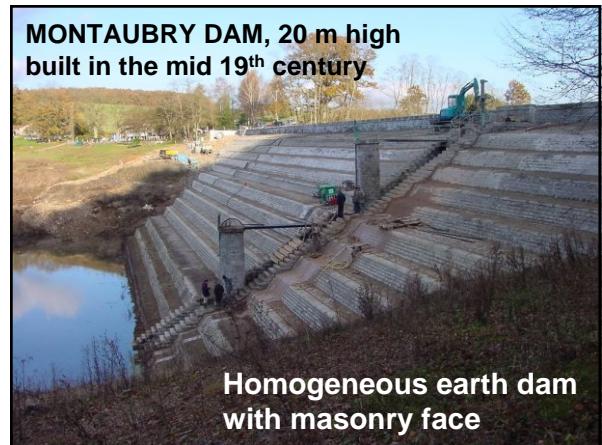
TEST RESULTS

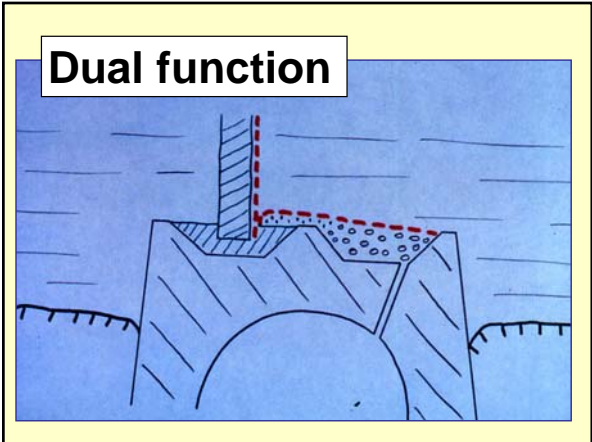
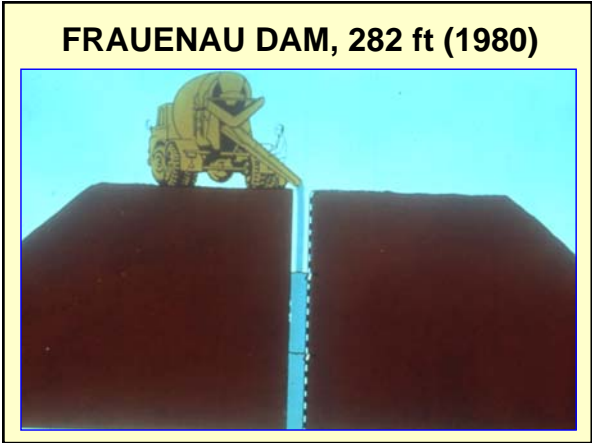
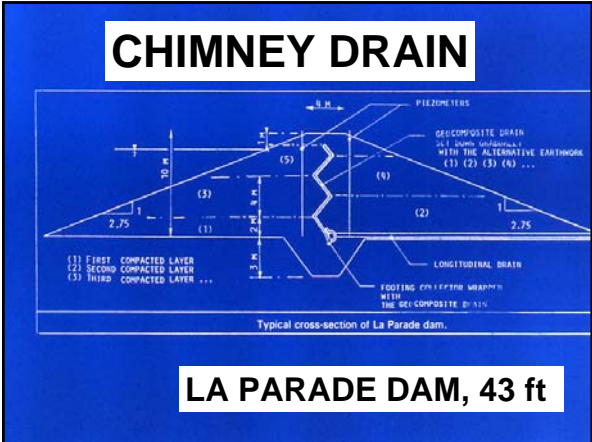
- Slight decrease in tensile strength from year 0 to year 6, and no change between year 6 and year 22.
- Same hydraulic conductivity in year 6 and year 22.
- Particles entrapped, less than 5% by weight.

VALCROS DAM

This outstanding performance can be explained.







VALCROS DAM

FILTER

The outstanding performance of Valcros Dam can be explained.

Fortunately, the Valcros Dam filter was not designed using classical filter criteria.

VALCROS DAM

PARTICLE SIZE DISTRIBUTION CURVE

COEFFICIENT OF UNIFORMITY
 $C_u = 90$ $C'_u = 53$

VALCROS DAM

TRADITIONAL FILTER RETENTION CRITERION
 $O_F < d_{85S}$

This problem with soils having a large coefficient of uniformity is known by geotechnical engineers.

Traditional solution: TRUNCATION

One may object that, so far, I used the retention criterion for cohesionless soils, whereas the soil in Valcros Dam has 30% particles smaller than 0.075 mm (#200 sieve).

Sherard proposed a retention criterion depending on the percentage of particles smaller than 0.075 mm. I used Sherard's criterion and obtained: 2.67 mm.

I used it again on the truncated particle size distribution curve. I found: 0.83 mm.

The soil in Valcros Dam seems to defy all retention criteria.

USE OF ADVANCED RETENTION CRITERION

$$O_F < \frac{18 d_{85S}}{(C'_u)^{1.7}}$$

| | |
|---|---|
| Application to the Valcros Dam soil With $d_{85S} = 6.4$ mm and $C'_u = 53$ $O_F < \frac{(18)(6.4)}{(53)^{1.7}} = 0.135$ mm | Application to the truncated particle size distribution curve With $d_{85S} = 1.8$ mm and $C'_u = 25$ $O_F < \frac{(18)(1.8)}{(25)^{1.7}} = 0.135$ mm |
|---|---|

This remarkable result confirms the validity of the advanced retention criterion.

We have found we needed a geotextile filter opening size of 0.135 mm.

What was the geotextile filter opening size at Valcros Dam?

It was not measured, but today (knowing the physical characteristics of the geotextile) we can calculate it using the following equation:

$$\frac{O_f}{d_f} = \frac{1}{\sqrt{1-n}} - 1 + \frac{10 n \rho d_f}{\mu}$$

The calculation gives: 0.099 mm, which is smaller than 0.135 mm.

0.1 mm measured in 1992

We can do even more.

Research in mid-1990s has shown that the risk of clogging of a geotextile filter is minimized if the number of constrictions is between 25 and 40.

$$m = \frac{\mu}{\rho d_f \sqrt{1-n}}$$

For the geotextile filter used at Valcros dam, the calculation gives 28.

CONCLUSION

FORTY YEARS OF EXPERIENCE

GEOSYNTHETICS IN DAMS SUMMARY OF EXPERIENCE

| GEOMEMBRANES | | |
|--------------------|----------|--------|
| • Rubber | 46 years | 106 ft |
| • Bituminous | 27 years | 138 ft |
| • PVC | 27 years | 571 ft |
| • HDPE | 20 years | 121 ft |
| GEOTEXTILES | 35 years | 106 ft |

CONCLUSION

- Geomembranes are now well accepted and are the material of choice for waterproofing the upstream face of all types of dams.
- The function of geotextiles filters **inside dams** is more subtle than the function of geomembranes **on the upstream face of dams.**

CHALLENGES REGARDING GEOTEXTILE FILTERS IN DAMS

- The geosynthetics engineering community should better inform the dam/geotechnical community about its research and development on **geotextile filters.**
- The geosynthetics engineering community should provide information on **geotextile durability** that is as good as the information developed in the past two decades on geomembrane durability.

